



## DO NOT BREAK THE SEAL UNTIL YOU GO THROUGH THE FOLLOWING INSTRUCTIONS

## **QUESTION BOOKLET**

Name of Post -ASSISTANT ENGINEER (PUBLIC HEALTH) - Example 2011
Name of Subject - CIVIL ENGINEERING - PAPER - I

Roll No.			Booklet Series A	
(En	ter your Roll num	ber in the above space)		
Time Allow	ed: 2 Hour		<b>Max. Marks: - 200</b>	)

## **INSTRUCTIONS:**

- 1. Use only BLUE/BLACK Ball Point Pen.
- 2. All questions are COMPULSORY.
- 3. All questions carry equal marks. Each question carries two marks. There will be no negative marking.
- 4. Check the BOOKLET thoroughly.

IN CASE OF ANY DEFECT - MIS<mark>PRINT, MISSING Q</mark>UESTION/S, <u>GET THE BOOKLET CHANGED</u>. NO COMPLAINT SHALL BE ENTERTAINED AFTER THE TEST.

- 5. Before you mark the answer, read the instruction on the back page of the OMR sheet (answer sheet) as well as on the question booklet before attempting the questions and fill the particulars in the ANSWER SHEET carefully and correctly. Incomplete & Incorrect particulars may result in your answer sheet not being evaluated by the Computer.
- 6. There are <u>FIVE</u> options to each question.
- 7. After completing the test, handover the ANSWER SHEET to the Invigilator.
- 8. For Rough Work, Blank Sheet is provided at the end of the question booklet
- 9. Write the BOOKLET SERIES given at the TOP RIGHT HAND SIDE of the question booklet in the space provide in the answer sheet, by darkening the corresponding circles.



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- 1. If a system of forces A, B and C are in equilibrium, then magnitude of their resultant is equal to
  - (a) A + B + C

(b)  $A^2 + B^2 + C^2$ 

(c)  $\sqrt{(A^2+B^2+C^2)}$ 

(d) Zero

- (e) None of the above
- 2. The maximum number of unknown forces that can be determined in a concurrent force system under equilibrium is
  - (a) Zero

(b) 2

(c) 3

(d) 6

- (e) None of the above
- 3. The principle of superposition states that the total deflection of a structure under different sets of oads is equal to the sum of deflections under each set of loads acting separately on the structure if the loads are within,
  - (a) Elastic limit

- (b) Limit State
- (c) Proportionality limit without buckling
- (d) Elastic limit including buckling

- (e) None of the above
- 4. The curvature at any point along the curve representing the deformed shape of a beam is given by

(a) 
$$\frac{1}{E} = \frac{\frac{dy}{dx}}{\left[1 + \frac{d^2y}{dx^2}\right]^{\frac{1}{2}-2}}$$

(b) 
$$\frac{1}{R} = \frac{\pm \frac{d^2 y}{dx^2}}{\left[1 + \left(\frac{2y}{x_0}\right)^2\right]^{\frac{2}{3}/2}}$$

(c) 
$$\frac{1}{R} = \frac{\frac{d^2y}{dx^2}}{\left[\left(1 + \frac{d^2y}{dx^2}\right)^{\frac{1}{2}}\right]^2}$$

$$(d) \frac{1}{R} = \frac{\frac{dy}{dx}}{\left[1 + \frac{d^2y}{dx^2}\right]^2}$$

- (e) None of the above
- 5. If a cantilever beam of span L and flexural rigidity EI carries a moment M concentrated at the free end, the deflection at the end will be,

(a) 
$$\frac{ML}{24EI}$$

(b) 
$$\frac{ML}{17EI}$$

(c) 
$$\frac{ML}{6EI}$$

(d) 
$$\frac{ML^2}{2ET}$$

- (e) None of the above
- 6. If a cantilever beam of length L and flexural rigidity EI is carrying a concentrated load P at the free end, the total strain energy will be,

(a) 
$$\frac{P^2L^3}{2EI}$$

(b) 
$$\frac{p^2 L^2}{3EI}$$
  
(d)  $\frac{p^2 L^2}{12EI}$ 

$$(c) \frac{p^* L^*}{6EI}$$

$$(d) \frac{p^2 L^3}{12EI}$$

- (e) None of the above
- A uniform simply supported beam is subjected to a clock-wise moment M at the left end. The moment required at the right end of the beam so that the rotation of the right end is zero is equal to
  - (a) 2M

(c) M/2

(d) M/3

(e) None of the above



8. The fixed end moment M<sub>FAB</sub> for the beam shown below is

A 1		M		В
7	1.10	2	1./0	——E
-	L/Z		<u>L/2</u>	
				(b) M/

- (a) Zero
- (c) M/4
- (e) None of the above

(b) 2EI/L

The moment required to rotate the near end of a prismatic beam through unit angle, without translation, the far-

(d)M/8

(a) EI/L

9.

(c) 3EI/L

(d)4EI/L

(e) None of the above

end being fixed is

- The flexibility coefficient for shaft of length L and torsional rigidity GJ under torsion at mid-point is 10.
  - (a)  $L^{2}/6GJ$

(b) L/2GJ

(c) L/3GJ

(d) L/GJ

- (e) None of the above
- . 11. Generally the actions in a grid member are,
  - (a) Axial force, twisting moment and bending moment (b) Shear force, twisting moment and bending moment
  - (c) Axial force, shear force and bending moment
- (d) Shear force and bi-axial bending moment

- (e) None of the above
- The number of unknowns to be determined in the stiffness method is equal to 12.
  - (a) Static indeterminacy

- (b) Kinematic indeterminacy
- (c) Sum of static and kinematic indeterminac (d) Maximum of static indeterminacy and kinematic indeterminacy
- (e) None of the above
- The force required to produce a unit translation displacement (without rotation) of one-third point of a fixed beam 13. of span L and of uniform flexural rigidity EI is
  - (a)  $729EI/2L^3$

(b) 729EI/L<sup>3</sup>

(c)  $724EI/L^3$ 

(d)  $724EI/3L^3$ 

- (e) None of the above
- The assumption that plane section remains plane under pure bending which is valid for linear elastic material is 14. not exactly true for plastic bending
  - (a) True

- (c) True if the material is nonlinear elastic
- (d) True if the deformations are continuous

- (e) None of the above
- 15. Which of the following sections will have the maximum shape factor
  - (a) Square

(b) Circular

(c) Diamond

(d) Triangle

(e) None of the above





The moment capacity of a section at plastic hinge	is
(a) Zero (c) Twice of yield moment (c) Name of the above	<ul><li>(b) Yield moment</li><li>(d) Fully plastic moment</li></ul>
(e) None of the above	
The variation of influence line for the stress functi	on in a statically determinate structure is
(a) Linear	(b) Parabolic
	(d) Rectangular
(e) None of the above	
The area of the influence line diagram for the fixed	d end moment of a fixed beam of span L is
(a) $L^2/8$	(b) $L^2/12$
(c) $L^2/16$	(d) $L^2/24$
(e) None of the above	
, 8 1	form load w per unit norizontal length, the maximum bending
	(b) $wL^2/8$
	(d) Zero
(e) None of the above	
The maximum decim stress and the area of the co	nergte stress block at Illtimate Limit State of flexure in a singly
	nervete stress offor at offiniate Emilie state of florate in a singly
	(b) $0.362  f_{ck}$ and $0.447 f_{ck} b x_u$
(c) $0.67  f_{ck}$ and $0.447 f_{ck} b x_u$	(d) $0.87 f_{ck}$ and $0.362 f_{ck} bx_u$
(e) None of the above	
The concept of locating neutral axis as a centroida	laxis (in a reinforced concrete section under flexure) is
	(b) True for both linear and nonlinear material behaviors
(c) True for both elastic and plastic materials	(d) True for elastic materials only
(e) None of the above	
TI G 1 C C 250	and in the place according to IC 456, 2000 is
	(b) $0.85/f_v$ times the gross area of concrete
	(d) 0.6% of the gross area of concrete
(e) None of the above	(1) 1.1.1.1 2.1.1.1 3.1.1.1
The maximum permissible characters in a steel of	tructural member as per IS 800-1984 is
	(b) 0.66 f <sub>y</sub>
	(d) 0.75 f <sub>v</sub>
(e) None of the above	(d) 0.73 ly
	"I was to a washink in hailt in at the compart and free at the
	cantilever beam which is built-in at the support and free at the
	(b) 0.85L
	(d) 2L
(e) None of the above	(-)
	(a) Zero (c) Twice of yield moment (e) None of the above  The variation of influence line for the stress functi (a) Linear (c) Circular (e) None of the above  The area of the influence line diagram for the fixed (a) L²/8 (c) L²/16 (e) None of the above  A three-hinged parabolic arch is subjected to a unimoment is (a) wL²/4 (c) wL²/12 (e) None of the above  The maximum design stress and the area of the correinforced rectangular section are (a) 0.447 f <sub>ck</sub> and 0.362 f <sub>ck</sub> bx <sub>u</sub> (c) 0.67 f <sub>ck</sub> and 0.447 f <sub>ck</sub> bx <sub>u</sub> (e) None of the above  The concept of locating neutral axis as a centroida (a) True for WSM and not LSM (c) True for both elastic and plastic materials (e) None of the above  The minimum flexural reinforcement of Fe 250 ste (a) 0.0012 times gross area of concrete (c) 0.4/(0.87 f <sub>y</sub> ) times the gross area of concrete (e) None of the above  The maximum permissible shear stress in a steel ste (a) 0.6 f <sub>y</sub> (c) 0.45 f <sub>y</sub>



25.

	C/2010/03	4	Series – A
33.	Weep holes are provided in retaining walls and be (a) To drain off the water from the filling (c) To reduce the weight of the earth retained (e) None of the above	reast walls  (b) To ventilate the stone masonry  (d) To increase the compaction of the	earth retained
32.	A cavity wall is generally provided for  (a) Heat insulation (c) Prevention of dampness (e) None of the above	(b) Sound insulation (d) All the above	
31.	Black cotton soil is unsuitable for foundations bed  (a) Its bearing capacity is low  (b) Its Permeability is uncertain  (c) Its Particles are cohesive  (d) It undergoes volumetric changes due to change  (e) None of the above		
30.	Rapid hardening cement contains  (a) Tri-calcium silicate  (c) Tetra-calcium alumino ferrite  (e) None of the above	(b) Tri-calcium aluminate (d) Di-calcium silicate	
29.	Cast iron is used for  (a) Structural works in beams  (c) Columns and struts  (e) None of the above	(b) Small sized water pipes (d) a & c	
28.	Seasoning of timber is done for  (a) Increasing moisture content  (c) Increasing strength of timber  (e) None of the above	(b) Decreasing moisture content (d) Decreasing strength of timber	
27.	Refractory bricks resist  (a) High temperature  (c) dampness  (e) None of the above	(b) Chemical action (d) all the above	
26.	Granite is an  (a) Igneous rock  (c) Sedimentary rock  (e) None of the above	(b) Metamorphic rock (d) All the above	
	wind/earthquake forces is  (a) 180  (c) 300  (e) None of the above	(b) 250 (d) 350	

The maximum slenderness ratio of a member of a steel structure, subjected to compressive forces resulting from



34.	produced in the prismatic bar by its own weight, wi	
	(a) 1: n	(b) n; 1
	(c) $1:\frac{1}{n}$	(d) 1: $\sqrt{n}$
	(e) None of the above	
35.	If <b>Z</b> and <b>I</b> are the section modulus and moment of immoment <b>M</b> at a section are related by	nertia of the section, the shear force F and the bending
	(a) $F = \frac{My}{I}$	(b) $F = \frac{M}{z}$
	(c) $F = \frac{dM}{dx}$	(d) $F = \int M dx$
	(e) None of the above	
36.	Simple bending equation is	
	(a) $\frac{M}{I} = \frac{R}{E} = \frac{E}{r}$	(b) $\frac{I}{M} = \frac{E}{R} = \frac{Y}{F}$
	(c) $\frac{M}{l} = \frac{E}{R} = \frac{F}{Y}$	$(d) \frac{M}{I} = \frac{F}{E} = \frac{Y}{F}$
	(e) None of the above	
37.	The shear stress at any section of a shaft is maximu	m (b) At a distance r/2 from the center
	<ul><li>(a) At the center of the section</li><li>(c) At the top of the surface</li></ul>	(d) At a distance 1/2 from the center
	(e) None of the above	
38.	If a solid shaft is subjected to a torque T at its end s diameter of the shaft will be	uch that the maximum shear stress does not exceed $f_s$ , the
		(h) [167
	(a) 163° recognition of fig.	$(0)\sqrt{\pi f_3}$
	(c) $\sqrt{\frac{167}{\pi f_3}}$	$(d) \frac{g_T}{\pi f_3}$
	(e) None of the above	
39.	In a tension test, the yield stress is 30 N/mm <sup>2</sup> , the o	octahedral shear stress at the point is
	(a) $10\sqrt{2} \text{ N/mm}^2$	(b) $15\sqrt{2} \text{ N/mm}^2$
	(c) $20\sqrt{2}$ N/mm <sup>2</sup>	(d) $25\sqrt{2} \text{ N/mm}^2$
	(e) None of the above	
40.	For a channel section the shear center lies at a dista	
	(a) $\frac{bdz}{2I}$	$(b)\frac{a^{2}bz}{3I}$
	$(c) \stackrel{a^2}{\overset{a^2}{=}} b^2 \varepsilon$	$(d)\frac{db^2t}{st}$
	(e) None of the above	width of the flange and common thickness of flange and web

respectively)



41.



For a given material Young's modulus is 200 GN/m<sup>2</sup> and modulus of rigidity is 80 GN/m<sup>2</sup>. The value of Poisson's





49.	The ratio of the maximum deflections of a simply supported beam with central load W and of a cantilever of sam length and with a load W at its free end, is					
	(a) 1/8	<b>(b)</b> 1/10				
	(c) 1/12	(d) 1/16				
	(e) None of the above					
50.	The equivalent length of a column of length	h L having one end fixed and the other end free, is				
	(a) 2L	(b) L				
	(c) I <sub>2</sub> /2	(d) $L/\sqrt{2}$				
	(e) None of the above					
51.	1 1	Expansion joints are provided if the length of concrete structure exceeds				
	(a) 10m	(b) 15 m				
	(c) 35 m	(d) 45 m				
	(e) None of the above					
52.	The aggregate impact value of aggregate us	sed in				
	(a) Building concrete is less than 45	(b) Road pavement concrete is less than 30				
	(c) Runway concrete is less than 30	(d) All the above				
	(e) None of the above					
53.	Workability of concrete may be improved	by adding				
55.	(a) Fly ash	(b) Hydrated lime				
	(c) Calcium chloride	(d) All the above				
	(e) None of the above					
54.	In a singly reinforced beam, the effective d	lepth is measured from its extreme compression edge to				
	(a) Tensile edge	(b) Tensile reinforcement				
	(c) Neutral axis of the beam	(d) Longitudinal central axis				
	(e) None of the above					
55.	The anchorage value of a hook is assumed	16 times the diameter of the bar if the angle of the bend is				
	(a) 30 <sup>[[]</sup>	(b) 40 🗆				
	(c) 45 🗆	(d) All the above				
	(e) None of the above					
56.	The stresses developed in concrete and stee	el in reinforced concrete beam of 250 mm width and 700 mm effective				
50.	depth, are 6.25 MPa and 25 MPa respectively. If m=15, the depth of it neutral axis is					
	(a) 200 mm	(b) 250 mm				
	(c) 300 mm	(d) 400 mm				
	(e) None of the above					
57.		he ratio of its effective length and lateral dimension, exceeds				
	(a) 10	(b) 12				
	(c) 20	(d) 30				
	(e) None of the above					
58.	The ratio of the diameter of reinforcing bar					
	(a) ½	(b) 1/5				
	(c) 1/6	(d) 1/8				
	(e) None of the above					
	C/2010/03	7 Series – A				



59.

Diagonal tension in beam

C/2010/03

(a) Is maximum at neutral axis



	C/2010/03	8	Series – A
66.	The assumption that the plane sections normal (a) Only in working stress method of design (b) Only in limit state method of design (c) In both working stress and limit state method of design (d) Only in ultimate load method of design (e) None of the above	n ethods of design	ending is used
65.	The final deflection due to all loads includi the as-cast level of supports, roofs and all o (a) Span/350 (c) Span/250 (e) None of the above	(b) Span/300 (d) Span/200	^
64.	Design of one-way RC slabs for concentrate (a) Using Pigeaud's moment coefficients (b) Taking slab strip of unit width containir (c) Taking slab strip of width effective in re(d) Taking orthogonal slab strips of unit width (e) None of the above	ng the load esisting the load dth containing the load	rinkoga and measured from
63.	Deep beams are designed for  (a) Shear force only  (c) Both shear force and bending moment  (e) None of the above	(b) Bending moment only (d) Bearing	
	(c) $0.16\sqrt{f_{ek}}$ (e) None of the above	(d) 0.30 \( \frac{1}{\varphi k} \)	
	(a) $0.25\sqrt{f_{ek}}$	(b) $0.20\sqrt{f_{ck}}$ (d) $0.30\sqrt{f_{ck}}$	
62.	At limit state of collapse in shear in case of maximum principal tensile stress exceeds a	value of t <sub>t</sub> equal to	e concrete cracks when the
	(e) None of the above	•	
	(c) 15% and 15%	(d) 20% and 20%	
61.	Prestressing losses in post-tensioned and pre (a) 15% and 20%	e-tensioned beams are respectively (b) 20% and 15%	
60.	High carbon content in steel causes  (a) Decrease in tensile strength but increase in tensile strength but decrease in tensile strength but decrease in tensile strength and duct (d) Increase in both tensile strength and duct (e) None of the above	n ductility tility	
	<ul> <li>(a) Is maximum at neutral axis</li> <li>(b) Decreases below neutral axis and increas</li> <li>(c) Increases below the neutral axis and decreas</li> <li>(d) Remains same</li> <li>(e) None of the above</li> </ul>	es above the neutral axis eases above the neutral axis	



67.	The state of the two dimensional stresses acting on a concrete lamina consists of a direct tensile stress, $\sigma_x = 1.5$ N/mm <sup>2</sup> , and shear stress $\tau = 1.20$ N/mm <sup>2</sup> , which causes cracking of concrete. Then the tensile strength of concrete in N/mm <sup>2</sup> , is			
	(a) 1.50 (c) 2.17	(b) 2.08 (d) 2.29		
	(e) None of the above			
68.	The following two statements are made with r  I. Failure takes place by crushing o  II. The neutral axis moves up as the	reference to a simply supported under-reinfor f concrete before the steel has yielded load is increased beyond yielding of steel	ced RCC beam:	
	With reference to the above statement	nts, which of the following applies?		
	(a) Both the statements are false	(b) 1 <sup>st</sup> is true but 2 <sup>nd</sup> is false		
	<ul><li>(c) Both the statements are true</li><li>(e) None of the above</li></ul>	(d) 1 <sup>st</sup> is false but 2 <sup>nd</sup> is true		
69.	A simply supported prestressed concrete bear prestressed by horizontal cable tendons at a u cable tendons is 1000 kN. Neglect the self-we beam at transfer is  (a) Zero  (c) 11.11 N/mm²  (e) None of the above	niform eccentricity of 100 mm. The prestress eight of the beam. The maximum normal com  (b) 5.55 N/mm <sup>2</sup> (d) 15.68 N/mm <sup>2</sup>	apressive stress in the	
70.	Which of the following deformations are imp (a) Shear (c) Torsional (e) None of the above	oortant in case of deep beams when compared (b) Axial (d) Bearing	to flexure alone	
-1.	The losses in prestress in pre-tensioning system 1. Elastic deformation of concrete 2. Friction 3. Shrinkage and creep of concrete Select the correct answer using the codes (a) 1, 2 and 3 (c) 1 alone (e) None of the above			
-2.	The critical section for two-way shear of foot (a) Face of the column (c) Distance d/2 from the column face (e) None of the above (where d is the effective	(d) Distance <i>d</i> from the column face (d) Distance <i>2d</i> from the column face	<b>3</b>	
<b>-</b> 3.	A reinforced concrete beam is to be post-tensioning. The distance of the tendon f (a) Between d/5 and d/4 (c) Between d/4 and d/3 (e) None of the above  (where d is the depth of	(b) $< d/6$ (d) $> d/3$	relops at the time of	
	C/2010/03	9	Series – A	



74.	An RC short column with 300 mm x 300 mm so 20 mm diameter longitudinal bars of Fe 415 ste Ignoring the reduction in the area of concrete d	quare cross section is made of M20 concrete and has 4 number of el. It is under the action of a concentric axial compressive load. ue to steel bars, the ultimate axial load carrying capacity of the
	column is	(L) 1540 LM

(a) 1659 kN

(b) 1548 kN

(c) 1198 kN

(d) 1069 kN

(e) None of the above

For avoiding the limit state of collapse, the safety of RC structures is checked for appropriate load combinations of dead load (DL), imposed load or live load (LL), wind load (WL) and earthquake load (EL). Which of the 75. following load combinations is NOT considered?

(a) 0.9 DL + 1.5 WL

(b) 1.5 DL + 1.5 WL

(c) 1.5 DL + 1.5 WL + 1.5 EL

(d) 1. 2 DL + 1.2 IL + 1.2 WL

(e) None of the above

If 'b' is the width if plate and 'd' is the diameter of rivet, then the efficiency of a riveted joint having diamond 76. riveting is given by

(b)  $\frac{b-d}{d}$ (d)  $\frac{b-2d}{d}$ 

(e) None of the above

When the effect of wind or earthquake load is taken into account, the permissible stress as specified in rivets may 77. be increased by

(a) 33.33%

(b) 50%

(c) 10%

(d) 25%

(e) None of the above

A column base is subjected to moment. If the intensity of bearing pressure due to axial load is equal to stress due to moment, then the bearing pressure between the base and the concrete is 78.

(a) Uniform compression through out

(b) Zero at one end and compression at other end

(c) Tension at one end and compression at the other end

(d) Uniform tension through out

(e) None of the above

If a tension member is subjected to axial load and bending moment, then 79.

(a)  $\frac{\sigma_{at,cal}}{0.66 f_y} \stackrel{!}{=} \frac{\sigma_{bt,cal}}{0.6 f_y} \le 1$ 

(b)  $\frac{\sigma_{at,cal}}{0.6 f_y} + \frac{\sigma_{bt,cal}}{0.66 f_y} \le 1$ (d)  $\frac{\sigma_{at,cal}}{0.66 f_y} + \frac{\sigma_{bt,cal}}{0.6 f_y} \le 1.4$ 

(c)  $\frac{\sigma_{ct,cel}}{0.6 f_y} + \frac{\sigma_{bt,cel}}{0.66 f_y} \le 1.4$ 

(e) None of the above

Ratio of plastic section modulus to elastic section modulus for circular section is 80.

(a)  $\frac{14}{3\pi}$  (c)  $\frac{16}{3\pi}$ 

(e) None of the above

10

Series - A





81. The value of the collapse load for a fixed beam of span $l$ , plastic moment $M_p$ , subjected to a conce at the mid-span, will be		
	(a) $\frac{4M_B}{L}$	(b) <sup>5M</sup> <sub>p</sub>
	6M <sub>+</sub>	(b) $\frac{5M_p}{L}$ (d) $\frac{8M_p}{L}$
	$(c) \frac{6M_{\rm p}}{L}$	$(d)$ $\frac{1}{L}$
	(e) None of the above	
82.	A cantilever steel beam of 3 m span carries a unit. The beam comprises ISLB 200 @198 N/m, flang $I_{yy} = 115.4 \text{ cm}^4$ . Bending and shear stresses in the	formly distributed load of 20 kN-m (inclusive of self weight). The 100 mm x 7.3 mm, web thickness 5.4 mm, $I_{xx} = 1696.6 \text{ mm}^4$ , the beam, respectively
	<ul> <li>(a) 530.47 N/mm² and 55.55 N/mm²</li> <li>(c) 132.62 N/mm² and 27.78 N/mm²</li> <li>(e) None of the above</li> </ul>	(b) 3899.48 N/mm <sup>2</sup> and 82.19 N/mm <sup>2</sup> (d) 1949.74 N/mm <sup>2</sup> and 41.10 N/mm <sup>2</sup>
83.	The problem of lateral buckling can arise only in (a) Moment of inertia about the bending axis larg (b) Moment of inertia about the bending axis sma (c) Fully supported compression flange (d) Concentric axial force (e) None of the above	ger than the other
84.	For a standard 45° fillet weld, the ratio of fillet to (a) 1:1 (c) √2:1 (c) None of the above	(b) 1:√2 (d) 2:1
85.	radius of gyration?  (a) Equal angles back to back  (c) Unequal legged angles with short legs back to  (e) None of the above	
86.	Angle of inclination of the lacing bar with the local 10° to 30°	ngitudinal axis <mark>of the co</mark> lumn should preferably be between (b) 30° to 40°
	(c) 40° to 70°	(d) 90°
	(e) None of the above	
87.	<ul><li>(a) 2.5% of the axial force in the member</li><li>(c) 10% of the axial force in the member</li><li>(e) None of the above</li></ul>	ll be designed to carry a transverse shear equal to (b) 5% of the axial force in the member (d) 20% of the axial force in the member
88.	As per IS: 800, for compression flange, the outst	tand of flange plates should not exceed
	(a) 12 t	(b) 16 t (d) 25 t
	(c) 20 t (e) None of the above	(4) 23 (



89.	The web crippling due to excessive bearing stres (a) Increasing the web thickness (c) increasing the length of the bearing plate (e) none of the above	(b) Providing suitable stiffend (d) All the above	ers
90.	Intermediate vertical stiffeners are provided in place (a) eliminate web buckling (c) transfer concentrated loads (e) None of the above	late girders to  (b) eliminate local buckling  (d) prevent excessive deflecti	ons
91.	The static theorem of plastic analysis satisfies (a) Equilibrium and mechanism conditions (c) Mechanism and plastic moment conditions (e) None of the above	(b) Equilibrium and plastic m (d) Equilibrium condition onl	oment conditions y
92.	The external wind pressure acting on a roof deper (a) Degree of permeability of roof	(b) Slope of roof	
	<ul><li>(c) Both (a) and (b)</li><li>(e) None of the above</li></ul>	(d) Direction of slope of roof	
93.	In the design of lacing system for a built-up steel is	column, the maximum allowable	slenderness ratio of lacing bar
	(a) 120	(b) 145	
	(c) 180	(d) 250	
	(e) None of the above		
94.	Which of the following loads are to be considered  1. Gravity loads 2. Lateral loads 3. Longitudinal loads 4. Wind loads Select the correct answer using the codes (a) 1 and 2 (c) 1 and 3 (e) None of the above	given below: (b) 1, 2 and 3 (d) 2, 3 and 4	
95.	A structure has two degrees of indeterminacy. The collapse		uld be formed at complete
	(a) Zero	(b) 1	
	(c) 2 (e) None of the above	(d) 3	
	(c) Induce of the above		
96.	A steel column in a multi-storied building carries a lacing. The lacing carries a load of		p of 2 ISMC 350 channels b
	(a) 125 N	(b) 12.5 N	
	(c) 3.125 N	(d) Zero	
	(e) None of the above		
	C/2010/03	12	Series – A





7

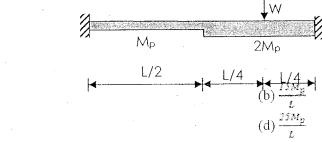
- 97. A member is subjected to axial compression. Effective length is 3000 mm. Size of the angle used is  $100 \times 100 \times 100$ . What is the maximum capacity (if  $f_v = 250 \text{ MPa}$ )
  - (a) 101.2 kN

(b) 81.7 kN

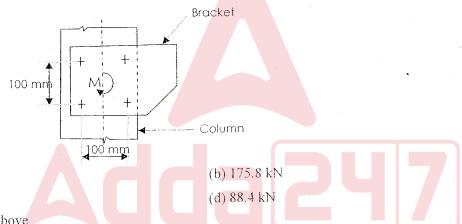
(c) 59.2 kN

(d) 95.1 kN

- (e) None of the above
- 98. Figure given below shows a fixed beam of steel. At the point of collapse, the value of the load W will be



- (e) None of the above
- 99. A moment M of magnitude 50 kN-m is transmitted to a column flange through a bracket by using four 20 mm diameter rivets as shown in the figure. The shear force induced in each rivet is



(c) 125 kN

(a) 250 kN

- (e) None of the above
- The tension member of a roof truss consists of two unequal angles 70 x 45 x 8 with the longer legs connected by 16 mm diameter rivets. If the angles are one on either side of the gusset plate, then the safe tension for the member will be (assuming permissible stress in tension = 150 MPa)
  - (a) 205 kN

(b) 215.4 kN

(c) 310.4 kN

(d) 320 kN

(e) None of the above